

The head in the inlet reservoir on the left is 20 m and the outlet reservoir on the right is 12m. Properties of the sand are: $K=1 \times 10^{-3}$ m/s. Draw and label a flow net. Calculate the discharge through the system using units of meters and seconds. What is the head at the location of the * at the top of the tank? What is the pressure at that location?

$\frac{8}{14} = 0.57$ contour interval
 $H = n\phi$
 $14 = nd$
 $\frac{h\phi}{nd} = 0.286$
 $\sim 18.23m$
 $p = \text{Head} - \text{Elevation} = 18.23m - 10m = 8.23m$

$q = KH \frac{n\phi}{nd} = 1 \times 10^{-3} \frac{m}{s} \frac{8m}{14} = 2.29 \times 10^{-3} \frac{m^2}{s}$
 $Q = \text{thickness} * q = 10m \cdot 2.29 \times 10^{-3} \frac{m^2}{s} = 2.29 \times 10^{-2} \frac{m^3}{s}$

K = 0.53m/day
Draw the flow net
Calculate Q = 0.53m/day 10m 2.5 / 10 ~ 1.3 m²/day
What is the maximum gradient? ~ 1
What are the head and pressure at the *? ~ 23.5m and 8m

We can use the flow net to identify areas where critical gradients may occur and determine the magnitude of the gradient at those locations

- equipotential lines parallel constant head boundaries
- flow lines parallel no-flow boundaries
- streamlines are perpendicular to equipotential lines
- equipotential lines are perpendicular to no-flow boundaries
- form squares by intersecting stream and equipotential lines

Stress caused in soil by flow = $j = i\gamma_w$
 If flow is upward, stress is resisted by weight of soil
 If j exceeds submerged weight of soil, soil will be uplifted

For uplift to occur $j > \gamma_{\text{submerged soil}} = \gamma_t - \gamma_w$
 where: γ_t - unit saturated weight of soil
 γ_w - unit weight of water
then for uplift to occur:

$$i\gamma_w > (\gamma_t - \gamma_w)$$

the critical gradient for uplift then is: $i_{\text{critical}} = \frac{\gamma_t - \gamma_w}{\gamma_w}$

What is the critical gradient for a soil with 30% porosity and a particle density of 2.65 g/cc (165 lb/ft³)?

$$\gamma_t = 0.7 (165 \text{ lb/ft}^3) + 0.3 (62.4 \text{ lb/ft}^3) = (134 \text{ lb/ft}^3)$$

$$i_{\text{critical}} = \frac{134 \text{ lb/ft}^3 - 62.4 \text{ lb/ft}^3}{62.4 \text{ lb/ft}^3} = 1.15$$

What is the flux under the sheet pile wall if $K=2\text{ft/day}$? Will piping occur? $Q = q_A n_f = KH \frac{n_f}{n_d}$

pond elev. 10ft

10ft standing water 20' 10' ground surface seepage

19' 18' 17' 16' 14' 13' 12' 11'

$Q = KH(n_f/n_d) = 2 \text{ ft/d } 10\text{ft } 4/10 = \sim 8 \text{ ft}^3/\text{day}$
 Using: $\gamma_t = 0.7 (165 \text{ lb/ft}^3) + 0.3 (62.4 \text{ lb/ft}^3) = (134 \text{ lb/ft}^3)$
 $i_{\text{critical}} = \frac{134 \text{ lb/ft}^3 - 62.4 \text{ lb/ft}^3}{62.4 \text{ lb/ft}^3} = 1.15$
 gradient is ~ 1.0 at the critical location, so it looks OK
 What could change that? How could you correct it?